



Fig. 12.3 The variation of the factor  $S_2$  and the wind velocity along the height of the building. (Assumptions made in the design shown in full lines.)

- 1st floor

$$29.313 \times (18 \times 18 / 2) = 4748.71 \text{ kNm}$$

- ground floor

$$1.1 \times (1269 / 10^3) \times 21 \times 21 \times 21 / 2 = 6463.2 \text{ kNm}$$

In the calculation the factor  $S_2$  has been kept constant (Fig. 12.3), which means the design will be a bit conservative. However, the reader can vary the  $S_2$  factor as given in Fig. 12.3 taken from Table 3 (CP 3) which means the wind speed will be variable depending on the height of the building.

### 12.5.3 Assumed section of wall resisting the wind moment

The flange which acts together with the web of I-section is the lesser of

- 12 times thickness of flange+thickness of web
- centre line to centre line of walls
- one-third of span

(a) Wall A

For wall A (Fig. 12.4), neglecting the outer skin of the cavity wall flange, the second moment of area is

$$\begin{aligned} I_A &= 2 \times \left( \frac{(0.1025)^3 \times 1.34}{12} + 0.1025 \times 1.34 \times (2.07)^2 \right) \\ &\quad + \frac{(4.045)^3 \times 0.1025}{12} \\ &= 1.169 \times 0.565 = 1.734 \text{ m}^4 \end{aligned}$$

(b) Wall B

The flange width which acts with channel section has been assumed as half of the I-section. For wall B (Fig. 12.5), neglecting the outer skin of the cavity wall flange,

$$\begin{aligned} I_B &= 2 \times \left( 0.67 + \frac{(0.1025)^3}{12} + 0.1025 \times 0.67 \times (2.07)^2 \right) \\ &\quad + 2 \times 0.1025 \times \frac{(4.045)^3}{12} \\ &= 0.571 \times 1.13 = 1.7 \text{ m}^4 \end{aligned}$$